Structure Erection Technology for PAR Modules of Ichthys Onshore LNG Project

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Abstract—The Ichthys LNG Project, one of the world's largest LNG developments, involves gas from the Ichthys Field in the Browse Basin off north-west Australia being exported to the onshore processing facilities in Darwin via an 889km pipeline. This project consists of 2 identical trains, one train of Ichthys PAR shall be fabricated as a whole structure and high accuracy shall be achieved in erection phase. It brings big challenge to erection work of these modules. Reasonable structure erection technology ensures good performance of fabrication activities. This paper presents mainly the structure erection technology for PAR modules of Ichthys Onshore LNG Project and also discusses special problems associated with the erection process including the erection sequence, the foundation settlement control, the method for dimensional control etc. The presented experience will benefit the construction of future modular construction.

Index Terms—Dimension control, erection, LNG, structures.

I. ABBREVIATION:

LNG	= Liquefied Natural Gas
PAU= Pr	e-assembled Unit
PAR	= Pre-assembled Rack
SPMT	= Self-Propelled Modular Trailers
COOEC	=China Offshore oil engineering co., LTD
WPS	=Welding Procedure Specification

II. INTRODUCTION

Modular construction is a well-proven technique for the Onshore LNG industry and is often preferred because of site conditions and cost saving [1].

Steel structures for LNG facility are usually separated into three types: 1. PAR: Modular structures that will be built in a construction yard out of site (pre-assembled Rack); 2. PAU: Modular structures that will be built in a construction yard out of site (pre-assembled Unit); "Stick-built" structures: built at site structures and 3. SPP: Pre-assembled structures at Site, and then transported by SPMT to their final position [2].

The Ichthys LNG Project, one of the world's largest LNG developments, involves gas from the Ichthys Field in the Browse Basin off north-west Australia being exported to the onshore processing facilities in Darwin via an 889km pipeline.

COOEC, as one of the Fabricators of Ichthys LNG Project is responsible for the construction of the PAR modules of Ichthys Project Onshore LNG Facilities.

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The process pipes for one train are connected integrally as a whole in final position. In order to ensure the accuracy of the pipe installation, it is required to adopt integratedconstruction method for each train, ie. all of the pipes in one train shall be installed the same as final status and then be cold cut between 2 adjacent modules after pressure test for the whole train and before transported by vessel. That means each train shall be fabricated as a whole structure and high accuracy shall be achieved in erection phase. The total weight of these LNG PAR modules is approximately 32000 tons. These LNG PAR modules mainly consist of 2 identical trains, each train includes 4 modules respectively, each module weights up to 6000 tons. Each train has a length of about 360m, a width of 48m as shown in Fig. 1.

Because the modules of 360m are fabricated in a whole, the elevation difference for the bottom surface of all of the columns shall be restricted within \pm 3mm during fabrication as per project requirement. It brought huge challenge to dimensional control. How to design the stools to minimize the negative impact of foundation settlements? How to achieve the target elevation difference for the bottom surface of all of the columns before erection? How to optimize the erection sequence? How to make sure the dimension requirement in the process of erection can meet the project requirement?

The following sections will answer these questions and present the structure erection technology successfully applied in PAR module fabrication.



III. FABRICATION STOOL DESIGN

During construction, the stools are used under the columns as temporary supports. The wind force shall be considered in stools design. For the temporary structure, the serving period of the stool is 2 years, the design period is 5 years. According to the design standard [3], the wind velocity shall be selected

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for five-year return period

Each module's support reactions shall be provided by detail design party. The maximum value of the force at the state of test and erection is used in the design of the stools and weighing stools.

The reaction of the column can be divided into two types, which are horizontal force and vertical force. Stools are to be designed according to the reactions and the foundation bearing capacity. Stability including overturning, sliding and soil pressure shall be checked in accordance with the design load and soil bearing capacity. Leg size of fillet weld chose 70% of thickness of bracing plate, which can meet the requirement of AISC [4]. Please refer to the Fig. 2 for the design method of the stools.



Fig. 2. Design method for stools.

IV. GROUND LEVELLING AND FABRICATION STOOL LAYOUT

Fabrication shall proceed on a flat level surface. Frequent level checks shall be performed. When any column or horizontal member settles more than tolerance, the settled column or horizontal member shall be shimmed back into a level plane. The differential elevations between module stool points shall be restricted to 3mm.

The detailed method to level the stool ground by placing concrete and grout under stools is as following:

Regolith shall be removed under the stool layout area (for concrete platform, the surface shall be cleaned) which shall be 100 mm protruded out from the stool base plate. The detail is shown in Fig. 3. Concrete shall be placed and cured first, certain gap between the top surface and the bottom surface of stool shall be achieved for ease of grout flowing, the stool shall be located on the target position by means of shimming. Concrete surface shall be blasted and wetted 24 hours before placing the grout to ensure a good bond between foundation concrete and grout material. Grouting and curing shall be performed after that.

The detailed work flow is as following,

- 1) Treat the construction area by dynamic compaction.
- 2) Clear and make the construction area flat prior to levelling.
- 3) Set up datum point. After that, all areas for stool laying shall be surveyed. The survey result will be the basis for choosing levelling method.
- 4) Clear the ground surface: Regolith shall be removed by digging shallow pit which shall be 100mm protruded out from the stool base plate. Clear the pit and compact the pit bottom. Re-measure according to the datum point, mark 0mm elevation at the location.
- 5) Arrange horizontal line in accordance with the 0mm elevation position to establish 0mm elevation datum as target surface.
- 6) Place support frame for concrete.

- 7) Prepare concrete: concrete strength shall not be less than C35.
- 8) Place concrete, vibrate and roll steel drum on its surface.
- 9) Concrete curing.
- 10) Blast and water soak concrete surface. All concrete surface shall be thoroughly cleaned and wetted prior to placing the grout to ensure a good bond between foundation concrete and grout material.
- 11) Place the stool on the target position by means of shimming. The level check of surface shall be performed using gradienter and the differential elevations between top surfaces of stools shall be restricted to 6mm. If the tolerance exceeds ±3mm, adjustment shall be made by means of shimming.
- 12) Place support frame for grout
- 13) Grout.
- 14) Perform maintenance





SECTION A-A Fig. 3. Details for levelling for stools.

Further adjustment method is as following:

- 15) The final level (with +/-3mm tolerance) can be further adjusted by using the shim plates inserted between module column base plate's bottom of steel (BOS) and the stool's top of steel (TOS).
- 16) Shim plates thickness shall be no greater than 10mm each and maximum of four pieces shim plates can be used at the same time providing 40mm maximum total shim plate height.
- 17) In no case the columns could contact with the unpainted shim plates/chocks. Unless otherwise noted, chocks/shim plates located at bottom of column base shall be blasted and painted. The edges distance between the column base to shim plates shall have at least 20mm. The edge distance from the shim plates to stool top of steel plate shall be at least 10mm. Bearing contact area between the column base plate and the shim plate shall be no less than 90% of the column base plate.
- 18) After field erection commences, workers will monitor the position of the stools periodically to ensure that they remain within tolerance. When there is settlement of stools outsides tolerance, it should be reported and remedial action shall be undertaken immediately.

V. SOLUTION TO FOUNDATION SETTLEMENT

The settlement monitoring and checks of level shall be made on a weekly basis. If unequal settlement occurs and structure inclines to one side, the erection work shall stop immediately. The settled section shall be brought back into level by jacking up the structure and then correct it by shimming. Misalignment shall be corrected at all stages of erection. The specific procedure is as specified in Fig. 4.

Requirement for jacking up operation is listed as following:

- 1) Unfixed and loose items in structure shall be removed before jacking up. The ground around shall be cleared.
- 2) Foundation bearing capacity must be calculated and verified to ensure that the foundation bearing capacity can meet requirements.
- 3) During the jacking up work, any obstacle within 10 m radius of work point is not permitted.
- 4) Personnel shall be kept away from work point and kept within safety zone.
- 5) Jacking work shall be carried out during daylight hours.

The method for adjustment after settlement is as following: Arrange one assembly stool underneath each column during fabrication. Arrange two jacks and stool adjacent to the column where unequal settlement occurs. Jack up the structure if the load of jack is within the rated capacity (Note: calculation shall be done to verify if member strength can withstand such load). In case of the load of jack exceeding the rated capacity, jacking up can be done by jack in conjunction with Synchronous Jacking System, then insert shim between the column and assembly stool to compensate the settlement. If equal settlement occurs and has no impact to erection and further work, more intensified investment and monitoring shall be conducted. Once inclining occurs, remedial action shall be undertaken immediately.

Note: It shall be ensured that the survey line does not move during erection.



Fig. 4. Work flow of solution to foundation settlement.

VI. FABRICATION SEQUENCE CONSIDERATION

For the general work flow chart for fabrication activity, please refer to Fig. 5.



It can be discerned from Fig. 5 that a lot of activities in other disciplines are involved in the fabrication process. Therefore a reasonable erection sequence shall be studied carefully to ensure high efficiency with high quality [5]–[7].

One typical module is as shown in Fig. 6. Each level of structure shall be divided in to several pallets according to the crane capacity and erection schedule. The Pallets break for one typical module is as shown in Fig. 7.



Fig. 6. Image of one typical module



Fig. 7. Pallets break for one typical module.

Each pallet will be lifted and erected by crane, each crane load shall be confirmed to be lower than the minimum safety work requirement. Gantry crane load shall be lower than 80% of its rated lifting capacity. Single crane load shall be lower than 90% of its rated lifting capacity. In the condition of two cranes associated loading, each crane load shall be lower than 80% of its rated lifting capacity. In the condition of more than two cranes associated loading, each crane load shall be lower than 75% of its rated lifting capacity [8]. The safety factor of the chosen sling shall be 4.0. The safety factor of the chosen shackling shall be 3.0. The actual lifting weight = theory weigh* Weight factor *Dynamic Amplificatory Contingency.

The lifting locations shall be studied well to ensure both safety and cost-saving [9].

Only certified rigging equipment and cranes can be used on the project. The dynamic amplificatory contingency of large structures conforms to requirements of DNV standard. DNV Dynamic Amplification Factors are shown as Table I [10].

Static-Hook Load	DAF Onshore	DAF Inshore	DAF offshore
50-100t	1.10	1.15	1.30
100-1000t	1.05	1.10	1.20
1000-2500t	1.05	1.05	1.15
>2500t	1.05	1.05	1.10

TABLE I: SELECTION OF DYNAMIC AMPLIFICATION FACTORS

VII. ALIGNMENT FOR PALLETS ERECTION

The alignment for pallet erection is very important for the dimensional control. Following method is developed to make sure the dimension can meet project requirement after erection.

Step 1: Before the alignment of upper pallet, the horizontal baseline of the lower pallet columns shall be adjusted accurately using gradienter and total station. Any inaccuracy out of tolerance shall be corrected by minor adjustment or relocating if necessary. The detail is as shown in Fig. 8.



Fig. 8. Adjust the horizontal baseline.



Fig. 9. Put down the upper pallet.

Step 2: Put down the upper pallet. There are many location points of incoming members already marked before alignment in the upper pallet, footprint areas to be ground to bright clean metal before assembly of pallets and six guide plates shall be tack welded under the chosen upper pallet primary beams, for details see Fig. 9. The purpose of these guide plates is to assist the crane to put down the pallet to the specific location.

Step 3: Install 3 Guide Plates with tapered lead in edges for weld access at 120 degree intervals onto the underside of the upper pallet nodes. For details see Fig. 10 and the Guide Plates are not welded to any component. The purpose of these plates is to adjust the Z-axes position of the upper pallet and to achieve the required root gap.



Fig. 10. Insert three guide plates.

Step 4: Install 3 spacer plates for weld access at 120 degree intervals onto the top of the tubular column. These plates shall project by 3- 5mm to maintain the root gap after erection, as per the applicable WPS, for details see Fig. 11.



Fig. 11. Insert three spacer plates.

Step 5: After the accurate orientation of the upper pallet, the upper beam and lower column shall be tack welded. For each welding position, at least 3 tack welds shall be applied and the length of each shall be over 50mm. For details see Fig. 12. Dimensional survey shall be conducted after tack welding.



Step 6: After tack welding, dimension inspection and visual inspection shall be implemented by QC. All of the Guide Plates shall be taken away by hammer knocking before

Step 7: Full welding shall be performed and cranes can be

release of cranes.

released after one third of all joints have been welded. For details see Fig.13.



Fig. 13. Full welding.

VIII. CONCLUSION

The fabrication work of PAR modules was challenging. Reasonable structure erection work-flow and sequences ensure good performance of fabrication activities. The structure erection technology is undoubtedly playing key roles for the successful execution of the project. Considerations to address ground bearing capacity limit, foundation settlement control, dimension control, erection sequence and alignment have been proven as good practices. In general, erection technology of the PAR modules will benefit future similar structure fabrication.

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He has already achieved much experience in field engineering and construction. He ever took part in international LNG projects as structure engineer such

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