

Simple Models of Foundation-Soil Interactions

Shi-Shuenn Chen and Jun-Yang Shi

Abstract—This study aims to develop a series of simplified mass-spring-dashpot model to simulate unbounded soil for a foundation-soil system subjected to vertical, horizontal, rocking and torsional motions. A group of equivalent models are established by using three equivalent criteria and the coupling of horizontal and rocking motions is also considered. An optimal equivalent model is then determined to represent the best simplified model. The dynamic responses of the foundation-soil system using the optimal equivalent model are compared with those obtained by the half-space theory and by the lumped-parameter models. Since the coupling of horizontal and rocking motions is adequately considered, the optimal equivalent model is found to have more accurate results than most existing models. Moreover, the proposed model is also applied to a time-history analysis for a building-soil interaction system subjected to horizontal excitations. This proposed method may be effectively applicable to practical soil-structure interaction problems.

Index Terms—Foundation vibration, lumped-parameter model, soil-structure interaction, seismic analysis

I. INTRODUCTION

The most commonly-used methods for dynamic soil-structure interaction (SSI) analysis are the complete method and the substructure method. The complete method features in analyzing the response of soil and structures simultaneously, which usually requires special boundaries to simulate the unbounded soil. In contrast, the substructure method features in analyzing soil and structures separately, which generally uses dynamic impedance functions to represent the soil behavior. The dynamic impedance function is the force-displacement relationship for interaction points between foundation and soil. However, since the dynamic impedances are frequency-dependent, they can not be used directly in time-domain structural analysis. Hence, simplified models with frequency-independent parameters are extensively developed for effectively simulating the unbounded soil in time domain.

One type of the simplified models is a lumped-parameter model, which comprises masses, springs, and dashpots. Through a curve-fitting technique or an optimization process, the modeling parameters of the lumped-parameter model are obtained by minimizing the discrepancy between the impedance functions for the model and that obtained from the rigorous theory. Then a lumped-parameter model is developed for a foundation-soil system while the foundation

is massless. Several studies [1]–[11] followed the conventional method addressed above to develop the simplified models. The conventional method used an optimization process to determine the unknown parameters of the lumped-parameter models so that the accuracies of those models were affected by objective functions used in optimization. Since the dynamic impedances in low-to-medium frequencies are more important on the system response, Wolf [7] suggested that the objective functions need to be adapted to make the weight for the low frequencies higher than that for the high frequencies. However, the relation between the frequency and the weight was not clearly clarified. Barros and Luco [2] suggested that an iteration method be adopted to determine an appropriate weight in the objective function until the impedance function of the simplified model approaches theoretical solutions well. This approach is relatively time-consuming.

This paper aims to presents a new simplified model to simulate unbounded soil for rigid embedded foundations undergoing vertical, horizontal, rocking and torsional motions. Parameters used in the new model are more efficiently determined without lengthy optimization analysis. Moreover, an optimal simplified model is selected through minimizing an error function which is clearly addressed and easily evaluated more than that used in most of conventional methods. The dynamic responses of the foundation-soil system using the new simplified model are compared with those obtained by the half-space theory and by the existing lumped-parameter models to examine the accuracy of the new model. Moreover, a time-domain validation for the proposed model applied to a building-soil interaction system subjected to vertical and horizontal excitations is also provided.

II. THEORY OF EQUIVALENT MODELS

This paper mainly investigates the dynamic response of a rigid embedded foundation undergoing vertical, horizontal, rocking and torsional motions. A series of equivalent models shown in Fig. 1 is used to simulate the unbounded soil. For vertical and horizontal motions, a single degree-of-freedom (SDOF) model is used. For rocking and torsional motions, a 2-DOF model is considered. The arrangement of the equivalent model includes a lumped mass (or inertia), a spring, and a dashpot in each directions; and two vertical eccentricities to consider coupling effects. For a foundation-soil system excited by harmonic forces with different frequencies, a group of equivalent models are developed by considering three criteria. The first criterion is equivalent static response, which establishes an equivalence of static displacement between the equivalent model and the foundation-soil system subject to a static force. The second

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criterion is equivalent magnification factor, which establishes an equivalence of dynamic magnification factor (i.e., ratio of displacement amplitude to static response) between the equivalent model and the foundation-soil system subject to a harmonic force. The last one is equivalent dissipated energy, which establishes an equivalence of dynamic dissipated energy between the equivalent model and the foundation-soil system subject to the harmonic force. Following the theory of equivalent models, the modeling parameters can be derived for each direction of motions. These parameters are summarized in the following.

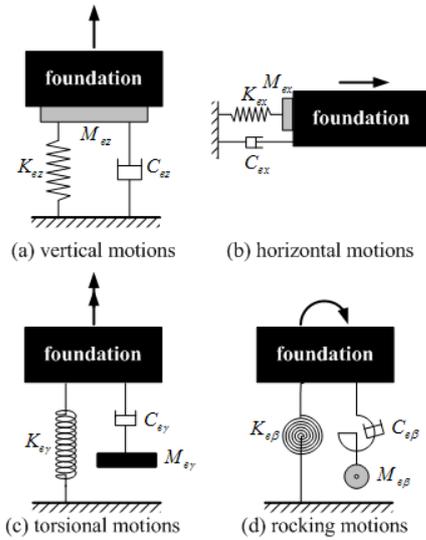


Fig. 1. A series of equivalent models.

For vertical motions, the modeling parameters with stiffness K_{ez} , damping coefficient C_{ez} and lumped mass M_{ez} can be shown as below.

$$K_{ez} = K_{sz}; C_{ez} = k_{ez}c_z\rho V_s R^2; M_{ez} = \frac{k_{ez}(1-k_z)}{a_0^2}\rho R^3 \quad (1)$$

where K_{sz} = static foundation stiffness of vertical motions; V_s = shear velocity of soil; R = characteristic length of the foundation; ρ = soil density; $k_{ez} = K_{ez}/(GR)$ = normalized static stiffness with a shear modulus G of soil; k_z and c_z = dimensionless stiffness and damping coefficients of vertical impedances, respectively; and $a_0 = \omega R/V_s$ = dimensionless frequency with a forcing frequency ω .

For horizontal motions, the modeling parameters with stiffness K_{ex} , damping coefficient C_{ex} and lumped mass M_{ex} can be similarly expressed as below.

$$K_{ex} = K_{sx}; C_{ex} = k_{ex}c_x\rho V_s R^2; M_{ex} = \frac{k_{ex}(1-k_x)}{a_0^2}\rho R^3 \quad (2)$$

where K_{sx} = static foundation stiffness of horizontal motions; $k_{ex} = K_{ex}/(GR)$ = normalized static stiffness; k_x and c_x = dimensionless stiffness and damping coefficients of horizontal impedances, respectively.

For torsional motions, the modeling parameters with stiffness K_{ey} , damping coefficient C_{ey} and lumped inertia M_{ey} can be written as below.

$$\begin{aligned} K_{ey} &= K_{sy} \\ C_{ey} &= \frac{k_{ey}[a_0^2c_\gamma^2 + (1-k_\gamma)^2]}{a_0^2c_\gamma} \rho V_s R^4 \\ M_{ey} &= \frac{k_{ey}[a_0^2c_\gamma^2 + (1-k_\gamma)^2]}{a_0^2(1-k_\gamma)} \rho R^5 \end{aligned} \quad (3)$$

where K_{sy} = static foundation stiffness of torsional motions; $k_{ey} = K_{ey}/(GR^2)$ = normalized static stiffness; k_γ and c_γ = dimensionless stiffness and damping coefficients of torsional impedances, respectively.

For rocking motions, the modeling parameters with stiffness $K_{e\beta}$, damping coefficient $C_{e\beta}$ and lumped inertia $M_{e\beta}$ can be similarly derived as follows.

$$\begin{aligned} K_{e\beta} &= K_{s\beta} \\ C_{e\beta} &= \frac{k_{e\beta}[a_0^2c_\beta^2 + (1-k_\beta)^2]}{a_0^2c_\beta} \rho V_s R^4 \\ M_{e\beta} &= \frac{k_{e\beta}[a_0^2c_\beta^2 + (1-k_\beta)^2]}{a_0^2(1-k_\beta)} \rho R^5 \end{aligned} \quad (4)$$

where $K_{s\beta}$ = static foundation stiffness of rocking motions; $k_{e\beta} = K_{e\beta}/(GR^2)$ = normalized static stiffness; k_β and c_β = dimensionless stiffness and damping coefficients of rocking impedances, respectively. For coupled horizontal and rocking motions, the modeling parameters shown in (2) and (4) are to be further modified by considering the coupling impedances in the dynamic equations of motions. More details are illustrated in the previous work [12].

Note that for a foundation-soil system excited by harmonic forces corresponding to N discrete frequencies, using the proposed method will result in N equivalent models. An optimal model is then determined from the established equivalent models to obtain the most accurate frequency-magnification response for the foundation-soil system subjected to harmonic forces; i.e. the optimal model has the minimum error of dynamic magnification factors. An error function used is defined as

$$\varepsilon_o = \sqrt{\sum_{j=1}^N \left(\frac{M_o - \bar{M}_o}{M_o} \right)^2} \times p_j \quad (5)$$

where M_o = dynamic magnification factor of the foundation-soil system; \bar{M}_o = dynamic magnification factor of the foundation with an equivalent model; the weighting $p_j = (M_o)_j$ corresponding to the j -th frequency point; N = number of frequency points and the subscript o denotes the direction of motion. This paper mainly investigates the dynamic response of a foundation-soil system using the optimal equivalent model to effectively simulate the unbounded soil.

III. VALIDATIONS ON FOUNDATION-SOIL SYSTEMS

Applications of the optimal equivalent model are to be investigated on the problem of foundation vibration in this

section. Consider a rigid square foundation embedded in a slightly damped half-space with damping ratios $\zeta_p = 0.0005$ and $\zeta_s = 0.001$. The embedment depth of foundation is assumed to be 1.5 times of characteristic length R (i.e., one half-side length) while the foundation mass is $5\rho R^3$. Suppose that the rigid foundation is harmonically subjected to a vertical force, horizontal force, rocking moment, and torsion, respectively, with varied frequencies. A series of vertical, horizontal, rocking, and torsional impedance function presented by Mita and Luco [13] is used in the proposed method to develop the optimal equivalent model to simulate the damped half-space. The dynamic magnification factors for a foundation-soil system using the optimal equivalent model are to be verified with those obtained by the half-space theory. The dynamic magnification factor is defined as the ratio of dynamic displacement amplitude to static response when the foundation subjected to an external harmonic force or moment. In addition, similar comparisons are also made for existing lumped parameter models. The analyzed results are displayed in Fig. 2. For vertical, horizontal, rocking, and torsional motions, observe in Fig. 2 that the frequency-magnification responses obtained from the optimal equivalent model consist well with the half-space solutions even as the dimensionless frequency increases. For vertical motions, the magnification factors analyzed by the optimal equivalent model consist very well with those obtained by Barros and Luco's model (2 DOFs and 5 parameters), especially for the peak responses. For torsional motions, the results analyzed by the Wu and Chen's model (3 DOFs and 9 parameters) underestimate the torsional magnification factors in low-frequency range but accurately estimate the high-frequency responses. By contrast, the torsional responses obtained by the optimal equivalent model consist well with the half-space solutions in all considered frequencies. For horizontal motions, Wu and Chen's model (1 DOF and 3 parameters) overestimates the horizontal magnification factors in comparison with the half-space solutions. For rocking motions, Wu and Chen's model (3 DOFs and 9 parameters) underestimates the rocking magnification factors especially for responses around the resonance frequency. This feature can be markedly observed for the rigid foundation with a deep embedment ratio or a large mass ratio. By contrast, good agreements are observed again between the results obtained by the optimal equivalent model and the half-space solutions. Hence, the optimal equivalent model may be more accurate than Wu and Chen's model due to the coupling effects on horizontal and rocking motions are well considered in the proposed method. The presented results also reveal that the rotational responses of embedded foundations subjected to a harmonic moment may be significantly underestimated without considering the coupled horizontal and rocking motions. The optimal equivalent model developed may also effectively simulate the uniform half-space for the rigid embedded square foundation undergoing forced vibrations.

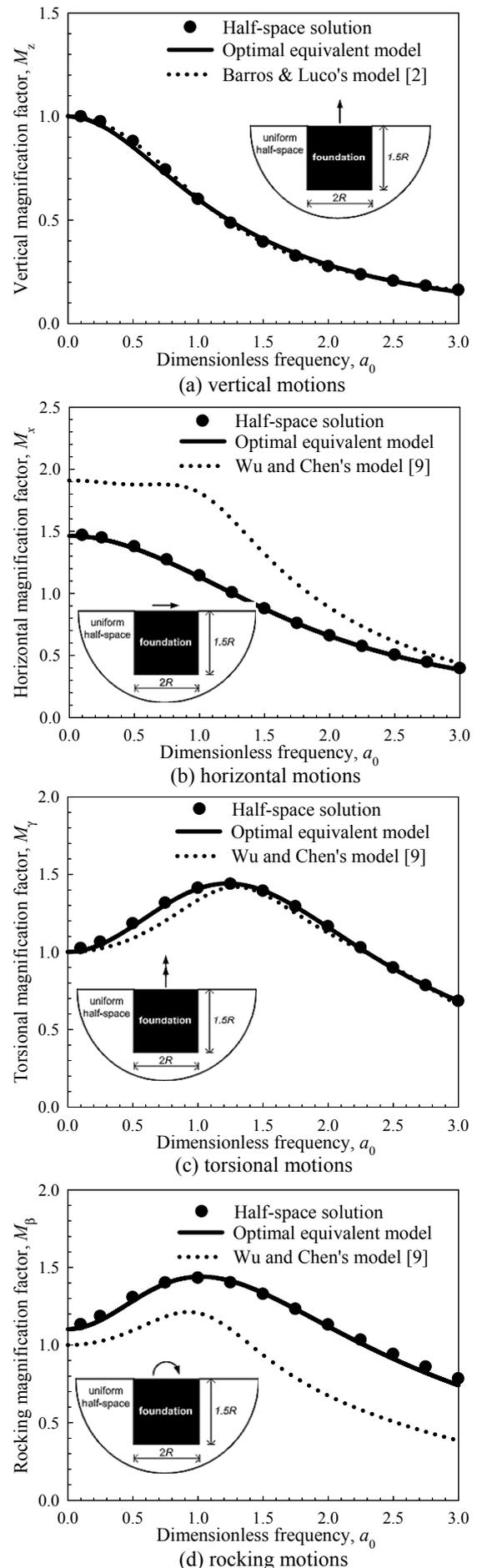


Fig. 2. Dynamic response of foundation-soil systems.

IV. VALIDATIONS ON BUILDING-SOIL SYSTEMS

Applications of the optimal equivalent model are to be investigated on the problem of seismic excitation in this section. Considered an idealized 3-story building (shown in Fig. 3) embedded in a damped half-space with uniform story stiffness and mass while the associated system parameters are listed in Table I. In the analytical models, the horizontal ground acceleration history recorded from Station TCU045 during 1999 Taiwan Chi-Chi earthquake is to be considered. As shown in Fig. 4, the responses of a simplified building-soil system using the optimal equivalent model are to be obtained by a modal analysis method in the time domain, which are accomplished by a commercial program, *SAP2000* [14]. Moreover, the responses of the real building-soil system are further analyzed by using a numerical program, *SASSI* [15]. This program *SASSI* features in using a special transmitting boundary to simulate the soil layer and a flexible volume method to solve dynamic soil-structure interaction problems. For the 3-storey structures, the horizontal acceleration history at the top floor is displayed in Fig. 5 while the associated floor response spectrum is shown in Fig. 6. Observe that the history responses of the simplified system mostly agree well with that of the real system. No marked differences on the accelerations or phases can be found. The spectral curves of the simplified system also consist well with that of the real system. In summary, for the building structure undergoing horizontal excitations, the seismic response of the simplified system agrees well with that of the real system.

TABLE I: SYSTEM PARAMETERS OF NUMERICAL EXAMPLES

| Zone | Parameter |
|--------------------|---|
| Uniform Half-space | Poisson's ratio: 0.25 |
| | Shear-wave velocity: 500 m/s |
| | Material density: 2 Mg/m ³ |
| | Damping ratio for <i>P</i> -wave: 0.0005 |
| | Damping ratio for <i>S</i> -wave: 0.001 |
| Square Foundation | Side length: 20 m |
| | Embedment depth: 5 m |
| | Foundation mass: 5000 Mg |
| | Foundation inertia: 177083 Mg-m ² |
| | Static stiffness: 4.16 × 10 ⁷ kN/m |
| Upper structure | Static stiffness: 5.58 × 10 ⁹ kN-m |
| | Storey mass: 2000 Mg |
| | Storey inertia: 66708 Mg-m ² |
| | Storey stiffness: 2.08 × 10 ⁷ kN/m |
| | Model damping ratio: 5% |

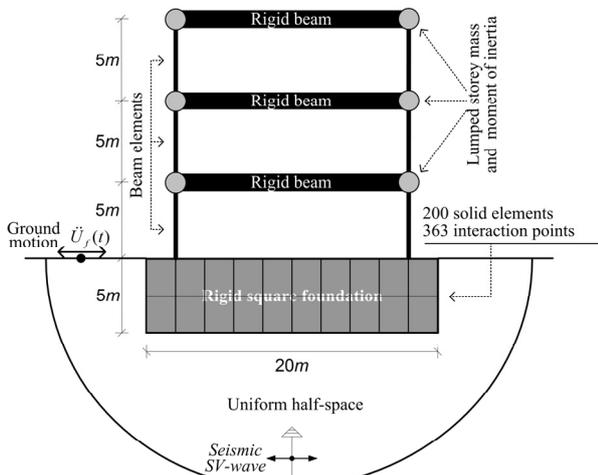


Fig. 3. Case study of an idealized three-story building: Real System

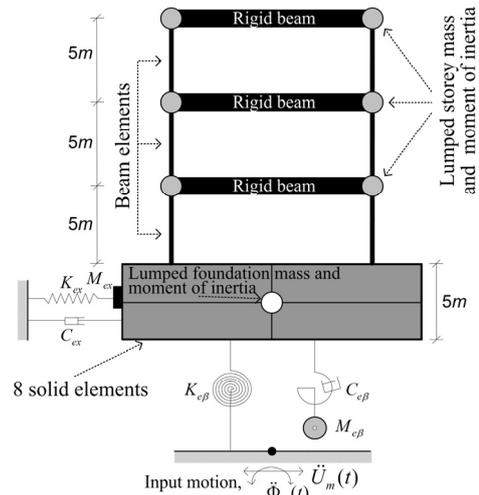


Fig. 4. Case study of an idealized three-story building: Simplified System

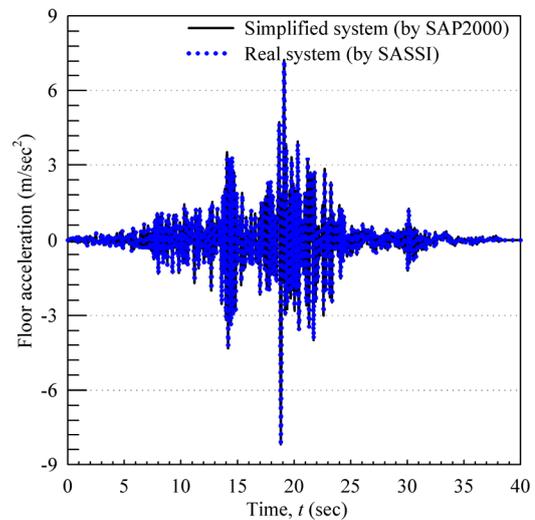


Fig. 5. Horizontal acceleration history of the top floor.

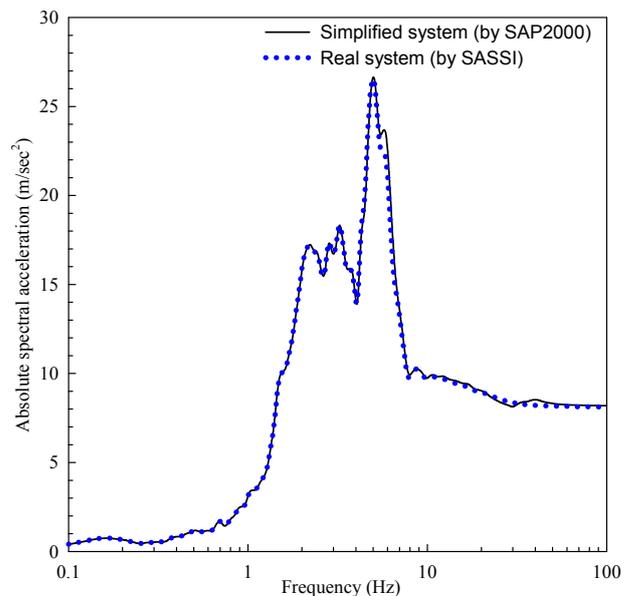


Fig. 6. Horizontal response spectrum of the top floor.

V. CONCLUSIONS

A simplified model is presented to simulate the unbounded soil for rigid embedded foundations. The modeling parameters are determined by the theory of equivalent model

without using lengthy optimization. For a foundation-soil system subjected to forced vibrations, the dynamic magnification responses analyzed by using the optimal equivalent model consist well with those obtained from the half-space theory and existing lumped-parameter models. For an idealized building structures subjected to horizontal excitations, the floor response of a simplified building-soil system using the proposed model may also agree with those obtained from the numerical program. This new method may be effectively applied to practical problems involving soil-structure interaction, such as machine foundation vibration and seismic structural analysis.

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